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Dynamic baselines for the detection of water quality impacts – the case of shale gas development†

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There is a need for the development of effective baselines against which the water quality impacts of new developments can be assessed. The specific conductance of flowback water from shale gas operations is typically many times the specific conductance of surface water and near-surface groundwater. This contrast in specific conductance means that specific conductance could be the ideal determinant for detecting water quality impacts from shale gas extraction. If specific conductance is to be used for detecting the impacts of shale gas operations, then a baseline of specific conductance in water bodies is required. Here, Bayesian hierarchical modelling of specific conductance was applied across English groundwater. The modelling used existing, spot-sampled data from the years 2000 to 2018 from 537 unique borehole locations. When the differences between boreholes was considered, then the approach was sufficiently sensitive to detect 1% mixing of fracking fluid in groundwater at a 95% confidence interval. The Bayesian hierarchical modelling maximises the return on public investment and provides a means by which future observations can be judged.

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Environmental significance

“Dynamic baselines for the detection of water quality impacts – the case of shale gas development”. This paper describes a new approach to defining environmental baselines. Environmental baselines are needed so as to be able to define whether an impact has, or has not, occurred with acceptable confidence. What is required is a probabilistic baseline which can predict the expected distribution at a monitoring location and so it is possible to judge whether a new observation is “unusual” at a given probability. In this study we use a Bayesian hierarchical analysis of the specific conductance of English groundwater to develop a baseline against which the impact of shale gas exploitation could be judged. The approach could detect 1% fracking fluid in groundwater at a 95% confidence.

1. Introduction

Many developed countries have extensive water quality monitoring networks that have been active for many years and so generating a large resource of water quality monitoring data, earliest river water quality monitoring starts in the nineteenth century;¹ and in the developed world this monitoring is now spatially extensive. However, monitoring data has often been acquired for very specific, sometimes local, purposes, and in the European Union this has often been focused upon the “good ecological status” dictated by the Water Framework Directive.^{2,3}

The use of these data in ways beyond those originally intended is attractive but can be problematic.⁴ Our aim was to determine whether this considerable quantity of data can be used not only to meet its original purpose but also aid in the protection and improvement of water resources from new challenges such as the development of a new onshore shale gas industry. One of the purposes of water quality monitoring is to assess whether activities have an adverse impact on a water body. To assess whether a human activity has an adverse impact or conversely demonstrate that no adverse impact has occurred, within an acceptable level of uncertainty, it is necessary to demonstrate that the activity (in this case shale gas exploitation) has changed the water quality above an acceptable limit or above a value that would have been expected without the activity present. In many cases, when the presence of pollutants is not overwhelming, then to demonstrate the presence or absence of an impact an effective baseline must exist, but such a baseline is not commonly available.

For a baseline, or indicator or change, to be of use in detecting and apportioning change or adverse impact there are

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a number of properties that a robust baseline should have. First, the monitored parameter should be related to the activities of concern, or provide an unrelated comparison for assurance purposes (e.g. a conservative marker), and the monitored parameter should be not related to other pollution sources. Second, the monitored parameter should be sensitive to the change with a large change in the magnitude for a small change in the activity. Third, any chosen water quality parameter should be a lead indicator of change, and in the case of water quality this may mean a determinand with little attenuation in its environment. It would be preferable that monitoring could provide early warning of any impact so that measures could be deployed in time to mitigate or avoid a problem. Finally, a larger sample size is preferable and that is more easily achieved if the measurement cost is low and readily deployable.

Shale gas exploitation is in its infancy in England and the potential impact on ground and surface water quality is a public concern.^{5,6} Extensive monitoring campaigns have been undertaken around a shale gas exploration site in Lancashire including the determination of baseline conditions for a wide variety of determinands.⁷ A number of technologies have been proposed directed at indicator substances for the monitoring of shale gas impacts on water resources, for example, measurement of dissolved CH₄,⁸ radium,⁹ barium and sulphate;¹⁰ strontium isotopes¹¹). However, by far the greatest contrast between flowback or produced water (*i.e.* water returning from shale gas operations) and shallow ground or surface water is their salinity, where salinity could be measured as total dissolved solids (TDS) or electrical conductivity. A review of the total dissolved solids (TDS) of shale gas flowback water from US shale gas operations showed that its salinity was between 0.67 and 10 times the TDS of seawater TDS ($\log \text{TDS seawater} < 4.6$) and far larger than the TDS of freshwater ($\log \text{TDS of freshwater} \sim 2.6$).¹² So far only one fracking operation has been conducted in the UK, for which flowback fluid composition has been measured (Preese Hall, Lancashire). The conductivity of the flowback fluid from the Preese Hall well was between 133 730 and 150 614 $\mu\text{S cm}^{-1}$.¹³ Measuring salinity, as electrical conductivity, requires no special equipment and is commonly and frequently measured in freshwater. Furthermore, the UK has been monitoring specific conductance (electrical conductivity of water corrected to a known temperature) across its range of fresh water bodies for decades. The availability and ease of collecting conductivity data are in contrast with some of the other indicators proposed for assessing shale gas, for example, the availability of strontium isotope data¹⁴ and the expense and facility required to collect further data.

These idealised characteristics for a good baseline indicator parameter mean that salinity, and its allied measures, specific conductance and TDS, are excellent prospects for detecting change in water quality from any developing shale gas industry. Measures of salinity show a high contrast between flowback fluids and freshwater environments and that this contrast is highly specific to shale gas. The high specificity and contrast between the salinity of the flowback fluids and the salinity of freshwater means that salinity could be a lead indicator of pollution incidents. What is more, high salinity water from

hydrocarbon exploitation has been shown to be toxic to exposed organisms.^{15,16} Shale gas sites would be expected to have a range of mitigation measures in place; however, Davies *et al.*¹⁷ showed that 6% of active, onshore oil wells in the UK have reported a pollution incident in the last decade. Clancy *et al.*¹⁸ showed, for Colorado, that for conventional oil exploitation sites, 0.02% of the produced water was spilt. Meanwhile in Alberta, Canada, in 2015 there were 113 incidents of spills of flowback and produced water documented.¹⁹ Boothroyd *et al.*²⁰ showed that 30% of decommissioned oil and gas wells showed significant CH₄ leaks even within 10 years of their decommissioning.

Although there are considerable numbers of specific conductance measurements available, these measurements were not collected with the purpose of creating a baseline for judging the impacts of a new industry. The government bodies responsible for water quality monitoring in England (The Environment Agency) are testing a range of statistical tools to use with monitoring data from shale sites but as yet they make no recommendations for indicator substances. Moreover, a simple, coherent method is required for the objective and transparent assessment of water quality monitoring. Worrall *et al.*²¹ proposed to use existing riverwater specific conductance data to develop a baseline against which the impacts of shale gas exploitation could be assessed using Bayesian generalised linear modelling. The proposed method had several innate advantages; the method was data-driven; flexible with respect to how the specific conductance data are represented; can use factorial (e.g. month of sampling) and include covariate information (e.g. land-use). Creating the statistical modelling within a Bayesian approach means that the approach creates a framework whereby all information has value. Equally, within this framework, new information can be added to update estimates and model outputs are a probability distribution which means that risk can be considered at all stages. The approach developed by Worrall *et al.*,²¹ creates a baseline which is dynamic in time and space as it predicts values for sites but updates with each new observation. However, the analysis of Worrall *et al.*²¹ showed that for river water the sampling frequency and the in-stream residence time compared to the likely volume spilt in acute incidents would mean that monitoring would be unlikely to detect all but the most catastrophic spills. Bayesian modelling approaches have been successfully developed and applied elsewhere for environmental management: for detecting dissolved CH₄,²² ecological risk assessment,²³ stream water quality;²⁴ impacts of climate change.²⁵

Therefore, the objective of this study was to develop a dynamic baseline for groundwater which is dynamic as it adapts in time and space as new data become available. Furthermore, the baseline, obtained by means of calculating the expected distribution of specific conductance in groundwater, will provide a probabilistic risk assessment of a pollution event. The approach of this study was to consider specific conductance in groundwater and use hierarchical Bayesian generalised linear modelling to predict the expected distribution at the time of sampling. Any sampled value can be compared to its predicted distribution to estimate the probability that an unusual event has occurred, *i.e.* there has been a pollution incident.



2. Methodology

2.1. Study sites

The study considered all the specific conductance at 25 °C measured in English groundwater wells and boreholes between the years 2000 and 2018 – no springs were included in this analysis. Only measurements that were from routine monitoring were included and data listed as being from pollution incidents were excluded as they will represent additional sampling at a higher frequency of times when higher specific conductance might be expected. Any boreholes that were listed as being part of or adjacent to landfill sites were removed. The data distribution of specific conductance was examined and values reported as having specific conductance lower than 40 $\mu\text{S cm}^{-1}$ were removed as these had specific conductance lower than that of rainfall²⁶ and such entries therefore were removed as being mis-entered data. Similarly, a QQ' plot was used to assess the distribution of the data and values above 6000 $\mu\text{S cm}^{-1}$ were excluded as being outliers – this represented the data from one site. The analytical methodology for specific conductance is outlined and controlled by the UK government's Standing Committee of Analysts (<http://standingcommitteeofanalysts.co.uk/>). Sites were retained within the study if they were sampled in at least two years during the period 2000 to 2018. This condition was used in part to restrict the number of sample sites that had to be considered and also to facilitate the estimation of interaction terms. Given the criteria for selection, there were 3370 observations from 537 unique locations that could be included in this analysis (Fig. 1).

2.2. Bayesian hierarchical generalised linear modelling

The estimation was based upon Bayesian hierarchical generalised linear modelling. Each data point (*i.e.*, specific conductance measurement – κ) within a generalised linear modelling

approach is assumed to be generated from a specific distribution within the exponential family of distribution. For the prediction of groundwater specific conductance ($\kappa - \mu\text{S cm}^{-1}$), the generalised linear model for the prediction of κ for site x at time t given a gamma distribution would be:

$$E(\kappa_{xt}) = \Gamma\left(\left(\frac{\mu_{xt}}{\sigma}\right)^2, \frac{\mu_{xt}}{\sigma^2}\right) \quad (\text{i})$$

$$\mu_{xt} = (\alpha_{xt}(\text{site, month}) + \beta_{xt}(\text{site, month})\Delta[\text{year}]_{xt}) \quad (\text{ii})$$

$$\Delta[\text{Year}]_{xt} = [\text{Year}]_{xt} - \overline{[\text{Year}]} \quad (\text{iii})$$

where $E(\kappa_{xt})$, the expected value of the groundwater specific conductance for site x at time t ($\mu\text{S cm}^{-1}$); site, the factor representing the different monitoring sites for which data were available; year, the different years of sampling; month, the different months of sampling. In this way β_{xt} is calculated for each monitoring site, for each month, across the record and represents the trend in the ground water specific conductance across the period for a particular site and month. Note that year was given as the difference from the mean of all the data (ΔYear) and in this way α_{xt} is not the y -intercept, *i.e.* the value of κ at year 0, rather α_{xt} is the expected value of κ at site x for the middle of the period of record. The approach of expressing year as the difference from the global mean value is so as to make estimation of more precise as α now sits in the middle of the observations rather than at one extreme as would be the case if was the y -intercept. Eqn (i) is a gamma regression; the gamma distribution rather than a normal distribution is used because it is not defined for values less than zero and so a negative κ is never predicted. However, to test the fit of the model proposed above, models were also fitted using both normal and log normal distributions and in the latter case the equations used are:

$$E(\kappa_{xt}) = N\left(\alpha_{xt}(\text{site, month}) + \beta_{xt}(\text{site, month})\Delta[\text{year}]_{xt}, \frac{1}{\sigma^2}\right) \quad (\text{iv})$$

$$E(\kappa_{xt}) = N\left(\alpha_{xt}(\text{site, month}) + \beta_{xt}(\text{site, month})(\log_e \Delta[\text{year}]_{xt}), \frac{1}{\sigma^2}\right) \quad (\text{v})$$

The approach given in eqn (i) through (v) is hierarchical because the model is established with a prior distribution on a prior distribution. In this case, the prior distribution on the terms α and β depends on the factors and each of these factors has its own prior distribution. The conceptual structure of the model is illustrated in Fig. 2.

Eqn (i) to (v) are expressed in terms of site and month as factors and year as a covariate. Year was considered as a covariate, which meant that for each of the factors (*e.g.* the difference between the months of sampling), the value of α and β could be estimated for each factor level included in a particular model. Furthermore, the inclusion of year as

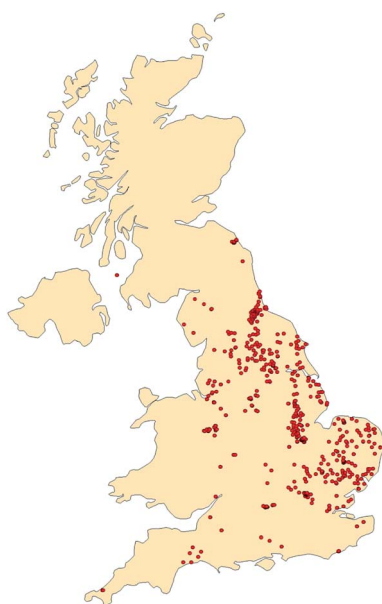


Fig. 1 The location of all the boreholes included in this study.



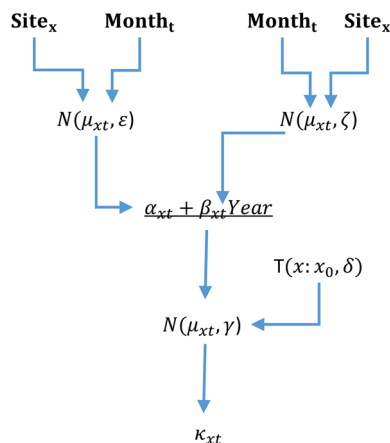


Fig. 2 Conceptual structure of the hierarchical model, where distribution is given as either $N()$ = normal or $T()$ = half T distribution. Each distribution is expressed as its expected value and a measure of spread. Factors are distinguished from covariates.

a covariate meant that predictions for future years could also be made for each of the factor level combinations. The site factor considers the difference between the monitoring sites and sites were included in the analysis if they were sampled in two or more years. Only sites with two or more years of sampling were retained as this facilitated the estimation of interaction terms, *i.e.* those terms that are the combination of factors. The month factor had 12 levels which is one for each calendar month.

As an alternative to the site factor, the aquifer class and aquifer type were considered. The aquifer class had seven classes defined by the British Geological Survey (BGS) and the classes are: 1A, 1B, 1C, 2A, 2B, 2C, and 3, where: 1 = intergranular aquifer; 2 = fractured aquifer; 3 = rocks with essentially no groundwater; furthermore, with A = highly productive aquifer; B = moderately productive; C = low productive aquifer.²⁷ The aquifer type is based on the geology that the borehole penetrates. The aquifer type was established from the British Geological Survey (BGS) aquifer designation map and the aquifer type was included if two or more sites sampled that particular aquifer geology; in all it was possible to include 27 different aquifer types (<https://www.bgs.ac.uk/products/hydrogeology/aquiferDesignation.html> – the dataset summarised by the aquifer class and type is given in Table S1†). The approach is hierarchical because the model is established with a prior distribution on a prior distribution. In this case the prior distribution on the trend over time, *e.g.* the terms α and β (eqn (i)), depends on the factors site, month, *etc.*, and each of these terms has its own prior distribution.

Markov Chain Monte Carlo (MCMC) simulation was used for the Bayesian analysis. The posterior distribution of the specific conductance was calculated using the Jags code called from R (version 4.0) using the R2Jags (version 0.6–1) library (R and JAGS code for the most complex model is included in the ESI†). An MCMC chain of length 10 000 iterations after a 2000 burn was used with samples saved every 10 cycles and with 3 chains.

The prior distribution for the values of β were set as normal distributions with a mean of zero so that both positive and negative trends in time were equally favoured at the outset, and standard deviation of the priors was set to be an uninformative prior and the likely small value of values of β , *e.g.* $N(0, 1000)$. The prior distributions for the values of α were also set as a normal distribution but with the mean set to be the mean of all the dataset and a standard deviation chosen to make negative values unlikely, *e.g.* $N(6, 0.25)$. The choice of such a distribution is justified because if values of β are small (*e.g.* influence of factors is weak), then α is the prediction of the specific conductance and thus should approximate the expected value of the distribution of the data as a whole. A half-t distribution was used for the prior distributions of the standard deviations for all terms as half-t distributions mean that a negative value of the standard deviation cannot occur, *e.g.* $\text{half}t(0, 5, 1)T(0)$.

The fit of model was tested by a number of approaches. Firstly, the adequacy of the MCMC process was assessed using \hat{R} , the convergence statistic, and values of $\hat{R} < 1.1$ were considered acceptable. If $\hat{R} > 1.1$, then the burn in process and number of iterations were increased. Second, for any factor, the 95% credible interval does not include zero, and going forward this is referred to as being a 95% probability of being significantly different from zero. Third, when a factor, interaction, or covariate is included, this caused total model deviance to decrease – deviance is a goodness of fit measure and is a generalization of the idea of using the sum of squares of residuals in ordinary least squares. Fourth, when an additional factor, interaction or covariate is included, there is a resulting decrease in the deviance information criterion (DIC). Because the inclusion of additional factors, covariates or interactions will increase the degrees of freedom of a model such inclusion would lead to a decrease in the total deviance of a model, and hence the need for another measure rather than just total deviance. The DIC accounts for the trade-off between the inclusion of more fitting parameters against the additional fit of the model and penalises for additional parameters relative to the fit of the model – DIC is the general case of the Akaike Information Criterion. Finally, the effective number of parameters (pD) was monitored, and it would be expected that as a factor or covariate was added to the model, then the number of effective parameters would likewise increase, and if pD did not increase with the inclusion of a factor or covariate then that parameter has no effect on the model and could be removed. Furthermore, that pD should be close to the ideal case if all parameters are contributing, and so therefore the calculated pD can be expressed as a percentage of that maximum possible – this value can never be greater than 100%. Finally, the fit of any model was judged using a posterior prediction check, *i.e.* the output of the model was plotted against the observed values and the fitted line between these two examined – it would be expected that a good fit model would give a 1 : 1 line between observed and posterior predicted values. Given the purpose of the model the balance of these criteria would be for a model that includes significant factors and gives the best possible fit as described by the lowest deviance given a reasonable fit across the range of observations shown in the posterior probability plots.



2.3. Model types

Three models were fitted

(i) Year, month and site factors – this model was used to give prediction at individual sites and was designed to understand the temporal trend at each site and assess the capacity for prediction and baseline development.

(ii) Year, month and aquifer class factors – the primary purpose of this model was to predict specific conductance for a previously unknown site. The aquifer into which any borehole will penetrate could be known before drilling, and so even if any previous sampling has not been included in the model development the question is whether it would be possible to predict specific conductance at sites not included in the analysis.

(iii) Year, month and aquifer type factors – as with the aquifer class we can consider a model based on the aquifer type as a means of predicting a specific conductance baseline for a site not previously sampled or not included in the model development.

2.4. Model verification and application

The data used for this study were groundwater data collected from the year 2000 to 2019. The models were developed on the groundwater data collected between 2000 and 2018, and to verify and further test the models, the models were used to predict the specific conductance for 2019. Given the factors included in the model, this prediction could be done for each borehole, aquifer class and aquifer type and for every month. These predictions were compared to the results of specific conductance in groundwater available for 2019.

As an example, the models were applied to the most and the least sampled sites and results shown over the study period.

2.5. The problem of the depth of sampling

The specific conductance of groundwater is expected to increase with the depth below the surface and therefore, any variation modelled could be due to variation in the depth that the sample came from. The depth of sampling is not recorded as part of the Environment Agency's monitoring; however, as part of separate investigations into the potential for geothermal energy in the UK a catalogue of depth constrained monitoring is available from Burley *et al.*²⁸ These data were used to test how much of the observed variation in specific conductance could be explained by the change in the depth of sampling. It should be noted that no groundwater body is delimited for water supply below 400 m in the UK.²⁹ No specific conductivity was reported within the

available geothermal database but a standard relationship between salinity and specific conductance was used for the conversion.³⁰

The relationship between the specific conductance and depth for English groundwater was used to examine whether variation in specific conductance observed between sites within this study could simply be ascribed to variation in the depth of sampling.

2.6. Comparative data

To give context to the specific conductance of English groundwater, the Environment Agency database was examined for the same time period to give the distribution of specific conductance for English river water, lake water and sewage treatment discharge. For English rainfall records from UK Precip-NET were used (<https://uk-air.defra.gov.uk/networks/network-info?view=precipnet>).

3. Results

Because the aquifer type was not defined for every borehole, the dataset when considering the aquifer type was slightly smaller with 3315 observations at 535 unique locations with 27 different aquifer types. The data have a median of 772.5 $\mu\text{S cm}^{-1}$ and a 95% percentile range of 221 to 2350 $\mu\text{S cm}^{-1}$. The comparative data for English river water, lake water, rain water and final sewage effluent (Table 1) show that groundwater does have a higher specific conductance

The distribution of the data by covariates and factors shows that there is a decline in the specific conductance of groundwater over time since 2000 (Fig. 3). Patterns of difference are less clear for the month and aquifer class factors (Fig. 4). For the geology, there are some clear distinctions with the lowest values in the Upper Greensand and highest in the Upper Devonian formations, although both of these aquifers had sample size of less than 6 samples (Table S1† – Fig. 5).

3.1. Statistical modelling

For all models the $\hat{R} < 1.01$ and so the fitting process for all models was deemed adequate. In all cases the best fit model was log normal for each model and the results of model fit relative to covariates and factors are detailed in Table 2. The pD will increase with the number of parameters included in the model, but the percentage of the expected pD may decrease as more parameters are included. The model with the most possible

Table 1 Summary of the specific conductance for comparative freshwater compartments

Type	Expected value	95% interval	N	Source
Groundwater	772.5	221–2350	3370	This study
Riverwater	633.5	95–1117	14 495	Worrall <i>et al.</i> (2019)
Lakewater	1243	45–7465	6967	EA WIMS
Rainwater	33	6–82	1484	UK PRECIP-NET
Final sewage effluent	1103.5	360–3739	12 245	EA WIMS



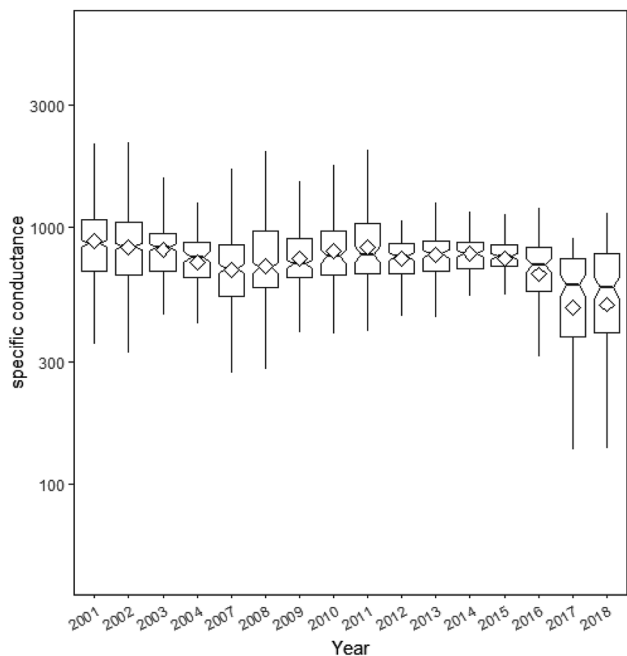


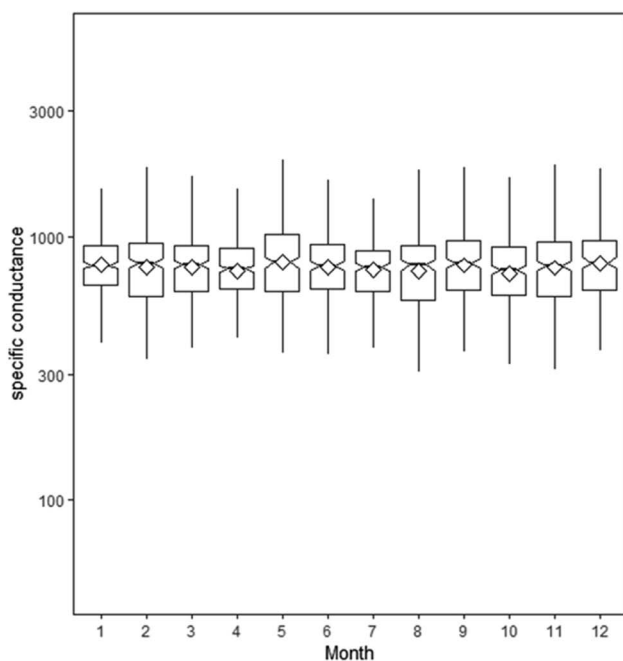
Fig. 3 The specific conductance relative to the year of sampling. The box represents the inter-quartile range (IQR), the line is the median value and the whiskers represent the values up to 1.5 times the respective IQR. The diamond represents the arithmetic mean of the data.

parameters (year + month + site) was the one with the lowest percentage of the expected pD. This low value of the percentage of the expected pD can be interpreted as that there being strong

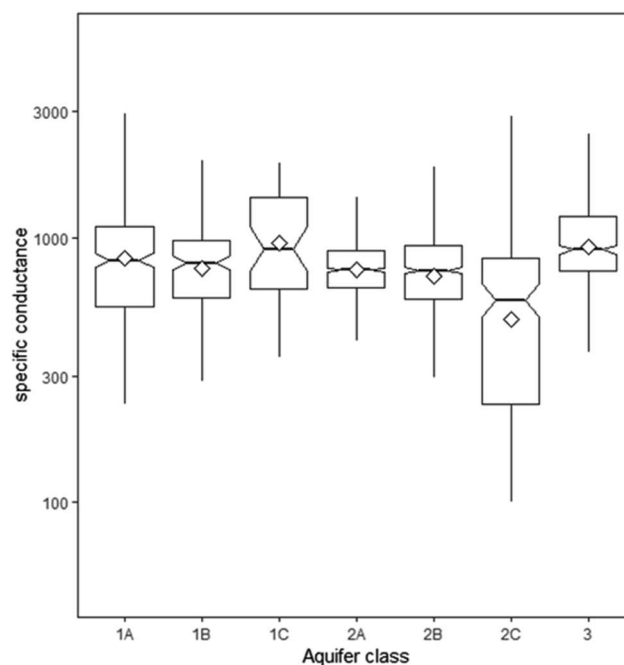
inter- and intra-annual relationships at some sites but by no means at all sites. The DIC value is at a minimum for the year + site model but increases for the model with the most parameters, *i.e.* year + month + site, implying that the most efficient model was year + site. However, model efficiency, as defined by DIC, may not be the most important criterion in terms of predicting specific conductance and the ability to fit the data accurately would be more important. With respect to deviance the best-fit model was the year + month + site model. The final test to be considered is the posterior prediction comparison (Fig. 6) and this suggests that the model year + month + site was the best predictor.

The slopes (β) estimated for each site shows that the mean slope estimates were negative for 497 out of the 535 sites with the remainder having positive estimates of the slope (Fig. 7). When the uncertainty in those estimates is considered, then it is possible to consider the chance that each of the slope estimates is zero, in which case at the 95% probability no site showed a positive slope and 7 showed a negative slope. The spatial distribution of the sites with significant slope, and indeed negative, slopes are all relatively coastal when compared to the distribution of all the boreholes in the study (Fig. 7). The site with the greatest significant slope would have a specific conductance decrease from $1685 \mu\text{S cm}^{-1}$ in the year 2000 to $572 \mu\text{S cm}^{-1}$ in 2018.

The slopes (β) estimated for each aquifer type shows that the mean slope estimates were negative for 20 out of the 27 aquifer types with the remainder having positive estimates of the slope. When the uncertainty in those estimates was considered, then 11 out of the 27 aquifer types had a 95% probability, while four



a)



b)

Fig. 4 The specific conductance relative to: (a) month of sampling and (b) the aquifer class. The box represents the inter-quartile range (IQR), the line is the median value and the whiskers represent the values up to 1.5 times the respective IQR. The diamond represents the arithmetic mean of the data.



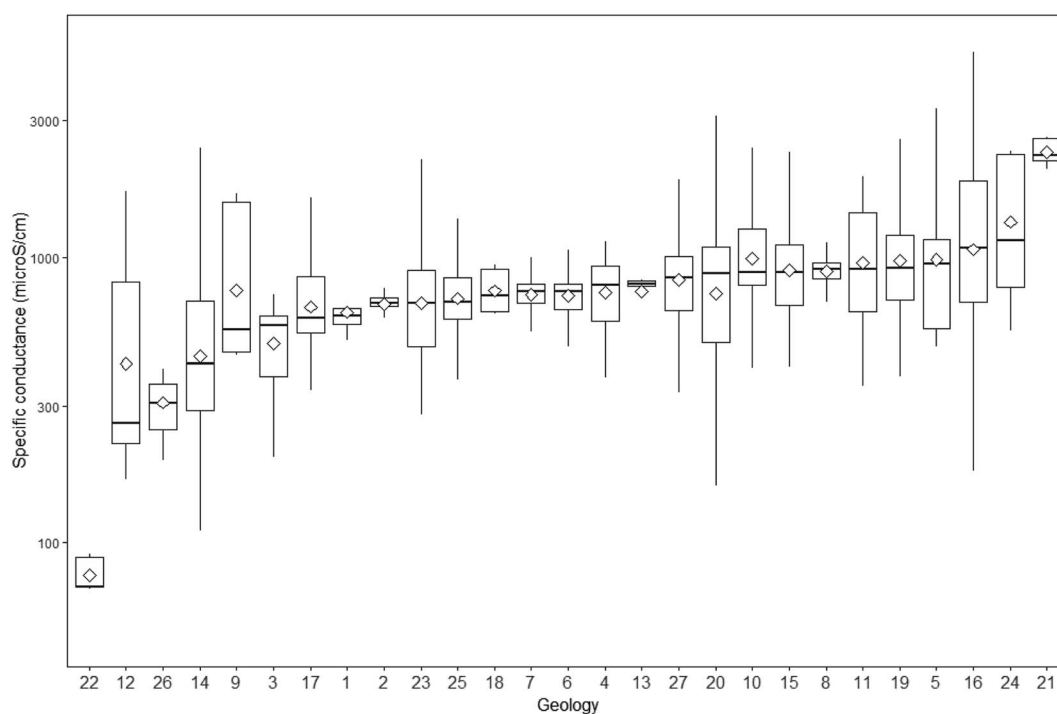


Fig. 5 The specific conductance relative to the aquifer type (geology). The box represents the inter-quartile range (IQR), the line is the median value and the whiskers represent the values up to 1.5 times the respective IQR. For clarity no marker of arithmetic mean was included. (1) Bowland High and Craven Group; (2) Corallian Group; (3) Dinantian rocks; (4) Fell Sandstone Group; (5) Gault Formation; (6) Great Oolite Group; (7) Grey Chalk Subgroup; (8) Inferior Oolite Group; (9) Inverclyde Group; (10) Kellaways and Oxford Clay Formation; (11) Lambeth Group; (12) Lias Group; (13) Lower Greensand; (14) Millstone Grit Group; (15) Neogene rocks; (16) Pennine Coal Measures; (17) Permian rocks; (18) Pridoli rocks; (19) Thames Group; (20) Triassic rocks; (21) Upper Devonian rocks; (22) Upper Greensand; (23) Wealden Group; (24) West Walton, Ampthill & Kimmeridge Clays; (25) White Chalk Subgroup; (26) Yoredale Group; (27) Zechstein Group.

Table 2 Fitting properties of the model combinations applied. The pD is expressed as both its absolute value and % of that, which could be expected if all new parameters included in the model were effective

Factors and covariates	pD (% expected)	DIC	Deviance
Year + month	21.5 (90)	4767	4745
Year + aquifer class	15.3 (95)	4318	4303
Year + month + aquifer class	201.9 (70)	4328	4126
Year + geology	60.2 (54)	3690	3630
Year + month + geology	545 (56)	4145	3600
Year + site	948 (59)	487	457
Year + month + site	4980 (38)	5344	364

aquifer types showed a significantly positive slope. The largest decline was for the Lias clay group and the greatest increase was for the Millstone Grit group.

The slopes estimated (β) for the aquifer classes showed that all seven aquifer classes had slopes significantly different from zero with five out seven aquifer classes showing a negative slope (aquifer classes: 1A, 1B, 1C, 2A and 2C), while two aquifer classes showed positive trends (aquifer classes: 2B and 3). The greatest increase was observed in aquifer class 3 and the largest decrease was estimated for aquifer class 2C.

The prediction of the model can be plotted both in terms of the expected value (the arithmetic mean as the fitted distribution was normal) and measures of the distribution across the UK (Fig. 8).

The distribution of expected values across the UK shows no distinctive patterns with no apparent pattern contrasting coastal with inland sites; no geographical trend (*e.g.* west vs. east); or upland vs. lowland sites. The pattern does indeed reflect the result in Fig. 6, *i.e.* the aquifer class and type are not as good predictors of groundwater specific conductance as site specific prediction.

3.2. Model verification application

The results for the most and the least sampled sites (Fig. 9) shows that in both cases the observed data are well within the envelope of the 95th percentile confidence interval predicted from the statistical modelling. Further, verification of the model comes from the comparison between the predicted and observed values for 2019, and this shows that all but 4 out of 29 possible observations were as predicted (Fig. 10). It is of course possible that the predicted distribution does not fit the observed because an unusual event has occurred at one of the sites. In each of the cases that could be considered in this study where the observed was outside the predicted distribution the observed was lower than the predicted distribution – therefore the modelling may be over-predicting values.

3.3. Problem of depth

From the Geothermal catalogue, the specific conductance is often greater at depth than at the surface (Fig. 10). At the



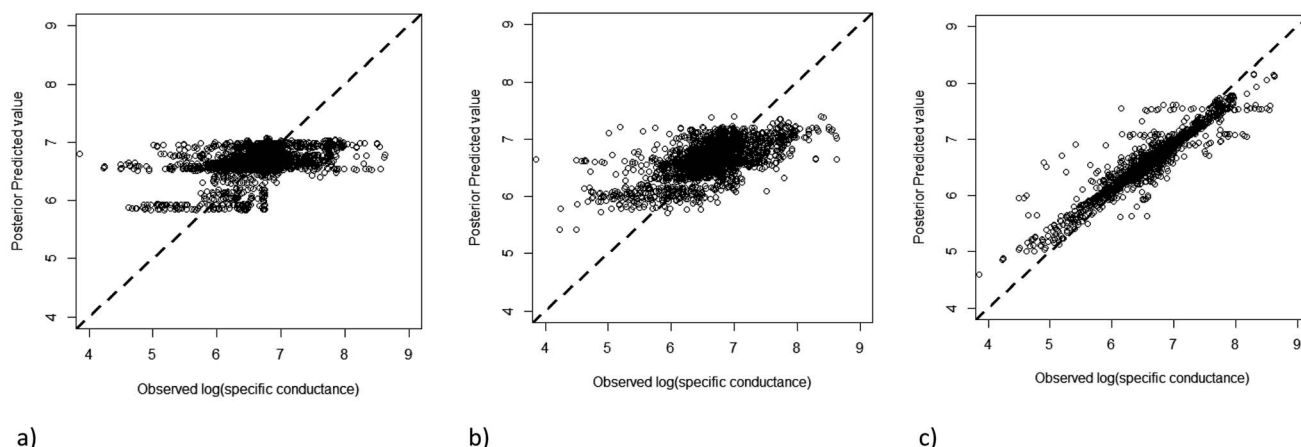


Fig. 6 Posterior prediction comparison for: (a) year + month + aquifer class; (b) year + month + aquifer type; (c) year + month + site.

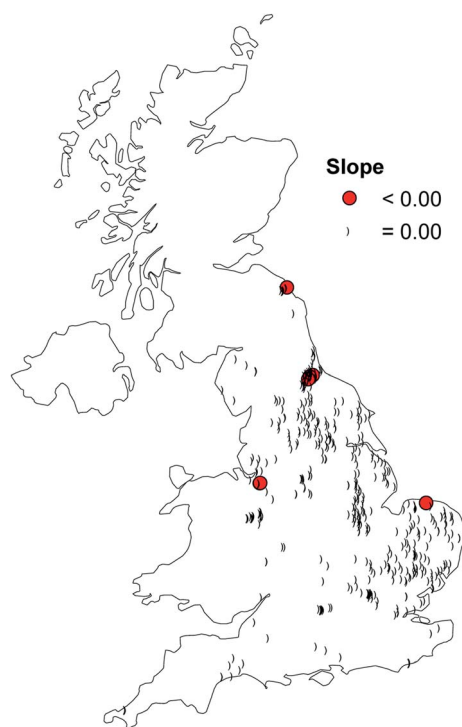


Fig. 7 The distribution of sites with significant mean slopes in comparison to the location of every monitored site in the study.

greatest depth sampled (2059 m) the specific conductance was $125\,108\ \mu\text{S cm}^{-1}$ which is a $\text{TDS} = 128\,714\ \text{mg L}^{-1}$ (Weyl equation³⁰). However, the sites in this study only show specific conductance in the box illustrated within Fig. 10 and in this box it would be difficult to suggest any relationship between the depth and specific conductance. Within the box drawn (as defined by the specific conductance within the dataset of this study), the depths of the samples from the UK Geothermal catalogue were up to 451 m below the surface. The lower extent of groundwater for water supply in the UK is often considered as 400 mbgl.²⁹

Over the entire range of depths sampled the best-fit equation was:

$$\log_e \kappa = 1.64 \log_e(\text{depth}) \quad n = 131, \quad r^2 = 0.51 \quad (0.14) \quad (\text{vi})$$

where depth = depth below ground surface (m). The value in the bracket is the standard error in the coefficient. Note that the constant term was not significant at 95% probability. However, another interpretation of Fig. 11 is that specific conductance has saturated at depth and that no value of specific conductance greater than $181\,895\ \mu\text{S cm}^{-1}$ ($\text{TDS} = 228\,728\ \text{mg L}^{-1}$) was ever observed.

For the range of specific conductance observed in this study (up to $2350\ \mu\text{S cm}^{-1}$), the relationship with the depth was not significant at a 95% probability of being different from zero, *i.e.* the inability to include the depth of sampling in this analysis was not critical to the result.

4. Discussion

Fig. 11 illustrates why analysing the specific conductance of groundwater will be useful for detecting water from depth. There have only been two fracking operations conducted in the UK at Preese Hall and Preston New Road, both in Lancashire, for which the conductivity of flowback fluid is known. The conductivity (not specific conductance) from the Preese Hall well varied from $133\,730$ and $150\,614\ \mu\text{S cm}^{-1}$.^{13,31} At the depth of fracking at Preese Hall, it would be expected that the specific conductance of the groundwater would be $450\,000\ \mu\text{S cm}^{-1}$ or even if Fig. 11 is viewed in linear as opposed to log–log space the predicted specific conductance would be $285\,000\ \mu\text{S cm}^{-1}$, *i.e.* this would suggest that there is mixing of the deep groundwater with either the fracking fluid itself or less saline water as the water raised back to the surface. The fracking fluid used at Preese Hall was a “slick water” fluid,³² a fluid that is predominantly composed of water and 1–2 additives (Table 3), usually a friction reducer and a chemical tracer.³³ At Preese Hall, 425 g of a “sodium salt” were used in $8399\ \text{m}^3$ of tap water, which would suggest that the TDS of the fracking fluid would be of the



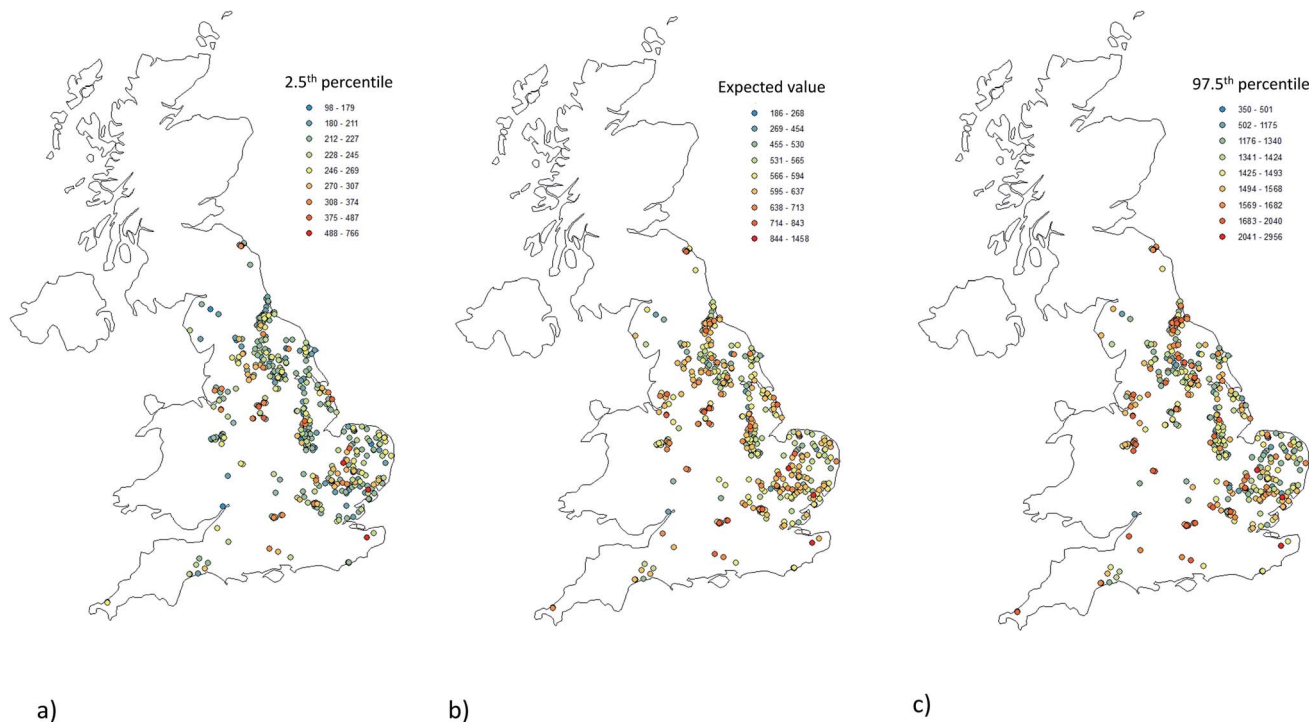


Fig. 8 The distribution of specific conductance at all study sites for the year 2018; (a) 2.5th percentile; (b) the expected value; (c) the 97.5th percentile. Note that the legends are not on the same scale.

order of 400 mg L^{-1} or $1122 \text{ } \mu\text{S cm}^{-1}$. Therefore, given the measured conductivity of the Preese Hall flowback fluid; the predicted specific conductance of the groundwater at the depth of fracking; and the specific conductance of the fracking fluids;

this would suggest that fracking fluid mixes with the groundwater at depth in 1 : 1 to 1 : 3 volume ratio.

Given the likely conductance of flowback fluid (for Preese Hall: $133\ 730$ and $150\ 614 \text{ } \mu\text{S cm}^{-1}$) and the predicted specific

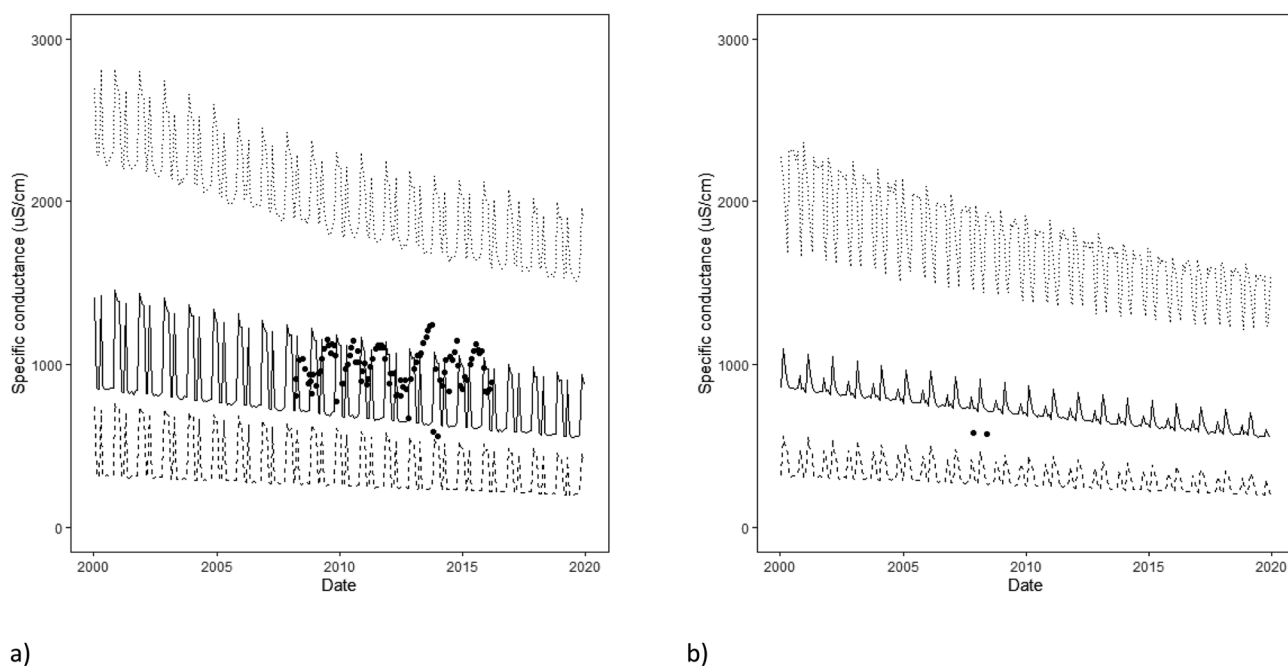


Fig. 9 The application of the modelling to: (a) Cold Bath Springs (British National Grid – ST531679 and (b) Airton Mills borehole (British National Grid – SD903592). The graphs show the 2.5th, expected value (mean) and 97.5th percentile predictions.



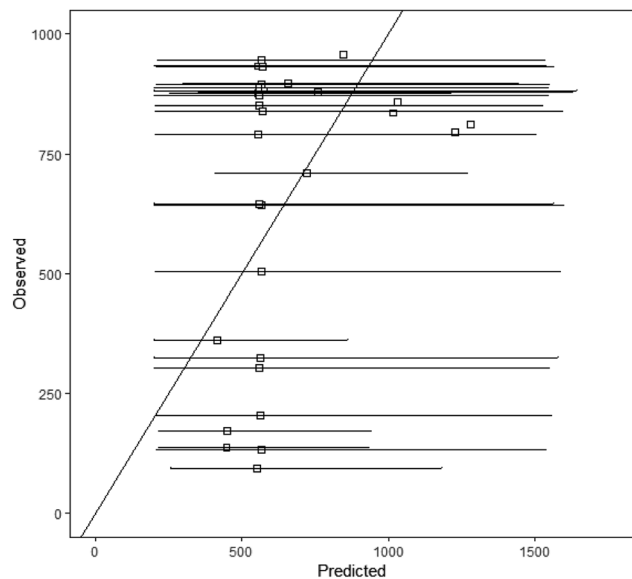


Fig. 10 The comparison of the predicted and observed specific conductance, which could be included in the modelling and for which measurements had been made in 2019.

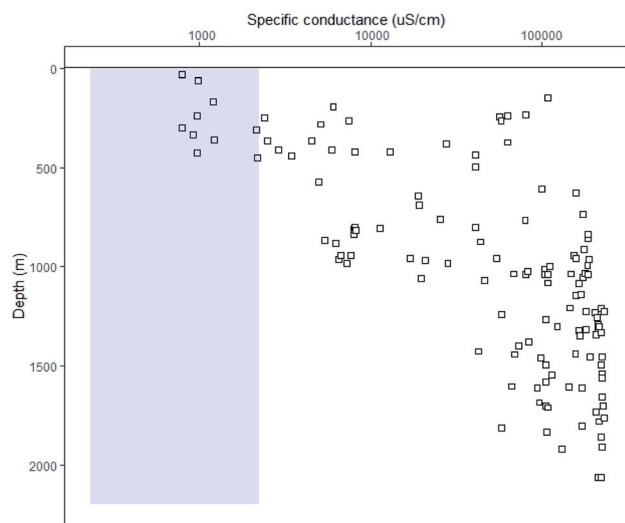


Fig. 11 The variation of specific conductance with the depth for data derived from Burley *et al.* (1984). The shaded box represents the values observed for surface water in England.

conductance of each monitored site (median = $696 \mu\text{S cm}^{-1}$, 95th percentile range of 404 to $1158 \mu\text{S cm}^{-1}$), it is possible to assess what amount of mixing of the flowback fluid would be

sufficient to increase the specific conductance of the local shallow groundwater to a value greater than would be expected at a given probability, for example:

$$\kappa_{97.5} < \phi\kappa_{\text{flowback}} + (1 - \phi)\kappa_{2.5} \quad (\text{vii})$$

where ϕ = the proportion of flowback fluid mixing into the surface groundwater at the site of concern; κ_x = specific conductance ($\mu\text{S cm}^{-1}$) with x as flowback (the κ of flowback fluid); 97.5 (the κ of the site at 97.5% probability); and 2.5 (the κ of the site at 2.5% probability). This test is conservative as it assumes that the mixing with the flowback fluid must be sufficient to cause the specific conductance of the shallow groundwater to rise from the 2.5th percentile of its predicted distribution to the 97.5th percentile of its predicted specific conductance distribution. Furthermore, if it is assumed that the κ of the flowback fluid is the lowest value recorded for Preese Hall, then the values of ϕ will be at the high end of the range. For the distribution predicted for each borehole for 2018, the values of ϕ range from 0.2 to 1.7% with a median of 0.9%. Clancy *et al.*¹⁸ has summarised the US literature on the volume of water required per fracking well and found that the volume ranged from 1500 to 45 000 m^3 , whilst Jiang *et al.*³⁴ noted that an average Marcellus well consumed 20 000 m^3 (with a range from 6700 to 33 000 m^3) of freshwater per well over its lifetime. The single well drilled in the UK at Preese Hall (Lancashire) required 8400 m^3 of water.³⁵ Therefore, for 8400 m^3 of flowback fluid and the median ϕ value predicted from eqn (vii), this volume would be sufficient to bring 906 000 m^3 of groundwater above the 97.5th percentile. Of course, it is unlikely that all 8400 m^3 of flowback would be released. Clancy *et al.*¹⁸ considered the water spill records kept by the Texas Railroad commission and the Colorado Oil and Gas Commission that the spill rate from produced waters was 0.1% per well per year meaning that nearer 906 m^3 of groundwater would be under threat, but this would be the volume under threat each year. In 2017 there were 18 655 licensed groundwater abstractions and the total groundwater abstracted was $2 \times 10^9 \text{ m}^3$, *i.e.* the average volume per abstraction is of the order of $1 \times 10^5 \text{ m}^3$ per year.

There is still little evidence that shale gas exploitation has actual significant impacts on groundwater resources. Fontenot *et al.*³⁶ did not find any significant increase in TDS in well water within 3 km of shale gas operations in the Barnett shale basin, Texas. Similarly, Warner *et al.*³⁷ found that the shallow groundwater composition in 127 boreholes of the Fayetteville shale gas basin, Arkansas, did not reflect the composition of the flowback and produced water from the shale gas operations. Harkness *et al.*³⁸ and Warner *et al.*³⁷ did suggest that there was

Table 3 Composition of hydraulic fracturing fluid for the Preese Hall 1A well in Lancashire, UK. Table was adapted from (Broderick *et al.*, 2011)

Additive	Type	Quantity	% by volume
Water	Local mains tap water	8399 m^3	97.93
Proppant	Glacial outwash sand	462.7 tonnes	2.02
Friction reducer	Polyacrylamide emulsion	3.7 m^3	0.043
Chemical tracer	Sodium salt	425 g	0.00043



evidence of contamination from leaks and spills originating on shale gas well pads. Wen *et al.*,³⁹ considered samples from 1384 boreholes and found that the higher dissolved CH₄ in 7 out of the 1384 boreholes could be associated with shale gas. Therefore, there is little published evidence that fluids injected at depth are actually causing a threat to shallow groundwater aquifers.

A simple approach to analyzing these data would have been to observe the distribution at one site and simply compare the newest observation with that distribution. Such a comparison would be limited by the size of the sample set at each site and for the sites used in this study the average sample size was only 6 samples. But such a comparison would not be fair as all the samples at the site might have been taken in summer months or perhaps at the beginning of the sampling period rather than in the most recent years. It would be better to compare the most recent observations with observations for the current year, even better with the most recent time step, *i.e.* for the same year, month, day of the month or hour of the day. Of course it is unlikely that there will be sufficient observations to give such a reasonable distribution for any month for any year at any site. So it would be useful if information from other sites could be drawn upon to make the most appropriate comparison possible; this then is what the approach of this study has achieved. By using all available information, the approach of this study could estimate a distribution of observations for every month and for every year at each site. An analogous, non-Bayesian approach might be weighted regression analysis.^{40,41} The Bayesian approach uses all available data to predict distributions at the sites of interest. The approach is a systematic and transparent approach to analysing data and provides a probability, with uncertainty, as to the nature of any observed data. Thus in turn the probability that any pollution has, or has not, occurred can be assessed. All risk assessment is actual a probability statement and the tools here use Bayesian approaches so all results will be a probability with an uncertainty. The Bayesian framework means that not only does the tool use all available data it also automatically updates as new information becomes available. A physical modelling approach would require more extensive data and observations to parameterise, calibrate and validate the physical model and so is not well suited to the type of dispersed data set that commonly arises from water quality monitoring. Conversely, the Bayesian approach developed here uses all the information and can return a result with clear uncertainty estimation.

For determinands such as specific conductance no specific legal standard exists within the UK or EU. For operators of works such as shale gas sites the review period for water quality monitoring is dictated in their license to operate, but, are data reviewed each time new data are produced and is the regulator informed if there is an issue? The regulator in the UK may be asked to report at any time to the secretary of state at the highest government level, although it is not clear how often this might occur. The approach presented here could be used for each new datapoint, *i.e.* the calculation and method presented would give the probability that a new observation is exceptional and not what should be expected.

Could it be possible to improve on the current approach? It is possible to include covariates and other factors within the analysis. This study has already included the aquifer class and type, but there could be a spatial correlation within the monitoring that could relate boreholes between aquifers. Such a spatial correlation in the observations is not included in this modelling approach. Qian *et al.*⁴² have developed a Bayesian hierarchical model for the calculation of nutrient loads in rivers that incorporated spatial correlation, but the spatial correlation was based on the flow through the river network which provides for one dimensional and directional correlation not appropriate for the three dimensions of the aquifers considered here.

Specific conductance could be expected to co-vary with some cations and anions and indeed different sources of water may have a similar range of specific conductance but a very different set of cations and anions that constitute that ionic strength. Other studies have considered other chemical indicators of pollution from unconventional hydrocarbons: Cl/Br;⁴³ Sr isotopes;⁴⁴ or the ratio (Ba + Sr)/Mg. Indeed, Wilson and Van Briesen⁴⁴ used Cl/Br ratios to detect shale gas fluids in surface water of the Monongahela river in Pennsylvania. However, a particular advantage of specific conductance is that it is regularly monitored whereas other determinands proposed in the literature have been far less frequently monitored.

This study has shown that between 2000 and 2018, specific conductance in English groundwater declined. Although only seven sites showed a significant decline the distribution of the slopes showed that 93% of sites had a negative slope and no sites had a significant increase. The salinisation of aquifers has been commonly reported across the globe, typically in coastal aquifers⁴⁵ and this can be due to overexploitation⁴⁶ or rising sea levels.⁴⁷ Not all salinification has occurred in coastal aquifers and Rivett *et al.*⁴⁸ have noted that in the UK we use between 1 and 3 Mtonnes of salt every year for road deicing and that this can be traced into aquifers. Indeed, many countries have noted deicing salt accumulating in aquifers (*e.g.* Canada⁴⁹) and Perera *et al.*⁵⁰ estimated that 19% of the applied road salt accumulated in the aquifer. These studies would suggest to expect salinisation in a country such as the UK where coastal aquifers are common and where there is an increasing demand for water with an expanding population. However, desalination was observed and never salinization. Han *et al.*⁴⁶ ascribed desalination in coastal karst aquifers to return flows from irrigation. Cloutier *et al.*⁵¹ gave an example of desalination as a recovery from a period of a high sea level during a glacial period as modern recharge flushes the seawater out. We would propose that the decline in groundwater specific conductance observed in this study could be due to changes in recharge patterns driven by climate change. If wetter winters caused an increase in the recharge of meteoritic water, then the specific conductance of the groundwater would decrease. Wen *et al.*³⁹ found significant declines in TDS since 1990 across 1384 in Pennsylvania and they ascribed these declines to declining atmospheric deposition and decreased inputs from the coal and steel industry.



5. Conclusions

This study has developed a Bayesian hierarchical model for determining the specific conductance of English groundwater. The model is able to estimate the distribution of specific conductance for any aquifer type, aquifer class or borehole that was included in the monitoring dataset. This estimation could be projected forward to predict distributions in the future, and furthermore, the Bayesian framework means that the probability estimation will continue to improve with all ongoing monitoring. The method was sensitive enough such that it could detect mixing of 1% of shale gas produced water into shallow groundwater.

Conflicts of interest

There are no conflicts to declare.

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